

*StructureWorks Precast –
Mathcad 13 integration overview.*



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Table of Contents

Introduction	3
User Specific Data Transfer Overview.....	3
Design binder.....	4
Variables.....	4
Dimensions.....	5
User Values	6
Feature Data	7
Integration UI	7
Integration Samples.....	8
Capacity Check.....	8
Automated Rebar Corbel Design.....	9
Automated Stair Design	10
Standard Variables.....	11
Top-Down Precast Dimensions.....	12
Conclusion	12
Appendix A.....	13
Rebar Corbel Plate Capacity Mathcad Worksheet	13
Appendix B.....	14
Automated Stair Unit Design Mathcad Worksheet	14

List of Figures

Figure 1 - SolidWorks® Design Binder	4
Figure 2 – Dimension Names.....	5
Figure 3 – User and Database Values.....	6
Figure 4 - StructureWorks Precast Mathcad Property Page	7
Figure 5 – Capacity Check Rebar Corbel Example.....	9
Figure 6 – Complete Automated Design Property Page.....	10
Figure 7 - Standard Automated Design Parameters.....	11
Figure 8 – Top-Down Reference Design Parameters.....	12

Introduction

Three dimensional computer-aided design (CAD) systems have been used in architectural applications for decades but traditionally avoided in precast construction due to the inability of associated software to be both cost-effective and feature-rich. Not only has CAD been slow to be adopted but integrated solutions for engineering design have been even slower to be developed let alone be adopted. While many other industries have had CAD integrated with analysis and design solutions, most of these revolve around Finite Element Analysis (FEA) applications.

Many of the manufactured concrete analysis and design solutions involve hand calculations, Microsoft® Excel files, or Mathcad™ worksheets. Integration of these tools for additional engineering automation has been very limited. The integration presented in this paper outlines some of the features and benefits of the integration between StructureWorks™ Precast and Mathcad™. This link bridges the gap between CAD modeling and engineering analysis and design.

StructureWorks has developed two approaches to this integration, User specific and StructureWorks specific. The user integration is the most robust and open while the StructureWorks specific is for a specific set of design or analysis tasks. The User Specific method is presented in this paper.

With this first in technology for this market place, **any** Mathcad™ 13 worksheet can be linked to any cad geometry. This link can be bi-directional allowing data to be sent between the two application or one way, allowing on one way data transfer. Linking both new and existing worksheets is a simple procedure and offers the ability to automate nearly every engineering design function. Now any product or component geometry can be used for analysis, or better yet the design of these components can be completely automated. This paper outlines and describes key features specific to this integration.

User Specific Data Transfer Overview

In summary a Mathcad sheet is attached to a StructureWorks Part or Assembly document through the SolidWorks® design binder. Variables in both documents are transferred between the two applications and the results can be stored in the Part, Assembly, and Mathcad files. In addition to dimensional values; features, database objects, and user inputs can be transferred between the files as well. There is visual notification if the model has changed from that last time it was designed or analyzed. Conditional alerts can be set up for user notification as well as return messages regarding the design or analysis. These messages are saved for additional review or for others to see when they open the file.

Design binder

The Design Binder is a folder in any SolidWorks file. The folder is a repository for the attachment of additional files. These files can be Embedded or linked Microsoft Word®, Microsoft Excel®, PDF, or other types of documents containing proposals, quotes, calculations, catalogs of purchased parts, design failure analyses, problem logs, reports, etc.

In every StructureWorks part or assembly there is a document called **Design Journal.doc** that is embedded in SolidWorks documents. The document has the following properties:

Appears in the Design Binder folder in the FeatureManager design tree.

- Is a embedded Microsoft Word® document.
- Resembles an engineering journal, with headings for File Name, Description, and Material. These fields are linked to document properties and updated whenever the journal is opened. File Name is the actual file name and Description and Material are SolidWorks document custom properties.
- Appears initially as <Empty>, a designation that disappears when you activate the journal. Can be deleted, in which case it is again listed as <Empty>.
- Accepts both text and embedded images.

For the StructureWorks Precast integration to be enabled we embed a Mathcad 13 worksheet or multiple worksheets.

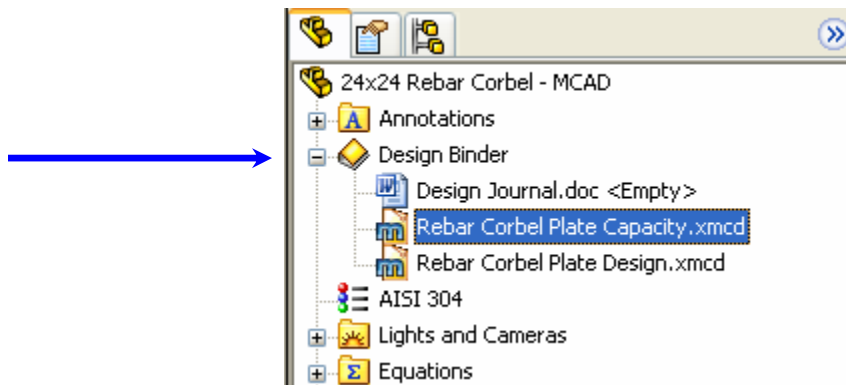


Figure 1 - SolidWorks® Design Binder

Any number of Mathcad Worksheet files can be place in the Design Binder for use in the analysis or design. These files are saved with the part file or can be saved to an alternate location.

Variables

StructureWorks Precast provides the user the ability to transfer variables from the StructureWorks Precast model to the Mathcad worksheet and in reverse. The method is simple

and straight forward. Any dimension or feature in the model can be named in a fashion than when an analysis of the variables is performed; StructureWorks traverses the names and passes Mathcad only the appropriate ones.

Dimensions

In Figure 2 a set of user dimensions have been placed on a sketch with reference to plate geometry. Both Bottom-Up Design¹ relationships and Top-Down Design² relationships can exist for reference values. When this plate is inserted with a connection or as an embed, if Top-Down relationships exist, any Top-Down relationship that modifies the plate geometry will automatically be reflected in these values.

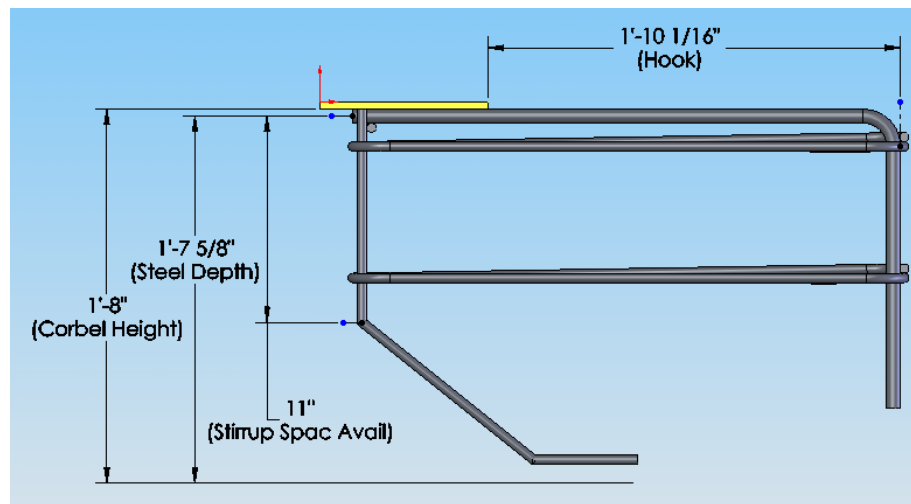


Figure 2 – Dimension Names

Figure 2 also illustrates the Dimension names, (Corbel Height), (Steel Depth), etc. of the dimension string. These names can be the default SolidWorks name or for convenience they can be user defined as in this case. These names represent the variables that will be passed to the Mathcad worksheet.

¹ Bottom-up Design - Bottom-up design is the traditional method. In bottom-up design, you create parts, insert them into an assembly, and mate them as required by your design. Bottom-up design is the preferred technique when you are using previously constructed, off-the-shelf parts. Source – *SolidWorks® 2006 Online User Guide*

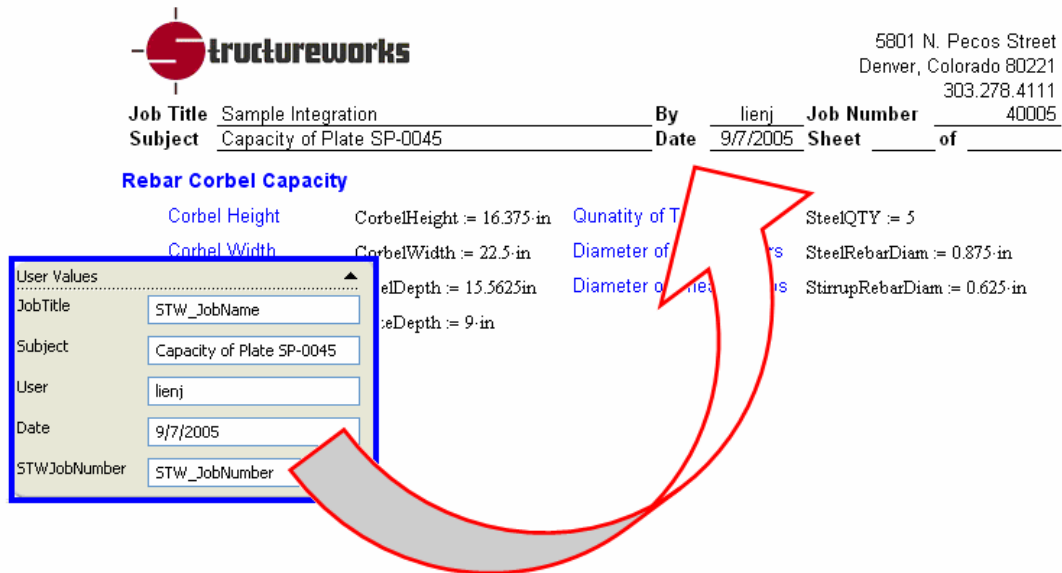
² Top-down Design - Top-down design is different because you start your work in the assembly. You can use the geometry of one part to help define other parts, or to create machined features that are added only after the parts are assembled. You can start with a layout sketch define fixed part locations, planes, and so on, then design the parts referencing these definitions. Source – *SolidWorks® 2006 Online User Guide*

In order for Mathcad to understand that a dimension in the StructureWorks model is to be used in the Mathcad worksheet, the Mathcad variable will need a **Tag**. For proper communication between the StructureWorks Model and the select Mathcad worksheet, the **Tag** name will be constructed of an *STW*- concatenated to the dimension name from the model, I.E. *STW-Corbel Height*.

The link is now complete. If the dimension in the model is *driving*, I.E. the dimensional value will modify geometry, then any changes in that value calculated by the Mathcad worksheet will modify the geometry in the StructureWorks model. If the Dimension in the model is *Driven*, I.E. the dimensional value will not control model geometry, then any changes in the value from StructureWorks Precast will modify the Mathcad worksheet.

User Values

In addition to dimensions, support of any variable either from the robust StructureWorks Database or User Defined can be sent either from StructureWorks Precast to Mathcad or in reverse. An example is automatic population of a calculation sheet header, the engineer can automatically fill out the Job number, Job Name, Design Engineer. The *StW_* in the name indicates a call to a query from the StructureWorks Precast Database.



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Job Title	Sample Integration	By	lienj	Job Number	40005
Subject	Capacity of Plate SP-0045	Date	9/7/2005	Sheet	of

Rebar Corbel Capacity

Corbel Height CorbelHeight := 16.375-in Quantity of T SteelQTY := 5

Corbel Width CorbelWidth := 22.5-in Diameter of SteelRebarDiam := 0.875-in

Corbel Depth := 15.5625in Diameter of StirrupRebarDiam := 0.625-in

Corbel Depth := 9-in

User Values

JobTitle STW_JobName

Subject Capacity of Plate SP-0045

User lienj

Date 9/7/2005

STWJobNumber STW_JobNumber

Figure 3 – User and Database Values

Recalling that StructureWorks Precast sits on Microsoft SQL Server, utilizing this connection, data from any modeled part or from any other department within the enterprise may be obtained.

Feature Data

In addition to dimensional values, an additional necessity is the ability to pass Feature data. Within StructureWorks Precast features represent a modeled part, a piece of rebar, a layer or multiple layers of mesh. Sending data to an analysis or design application for inclusion or exclusion of these features for simplifying the mathematical problem is very important. In addition to simplifying the design problem, one needs the ability to edit the features or Suppress and Un-suppress specific entities based on the design output, I.E adding shear rebar or subtracting a layer of mesh.

Integration UI

StructureWorks Precast is a fully integrated solution with the SolidWorks cad platform. Nearly all the StructureWorks Precast Graphical User Interface (GUI) utilizes the SolidWorks Heads-Up Display or Property Page. The StructureWorks Mathcad integration is no different.

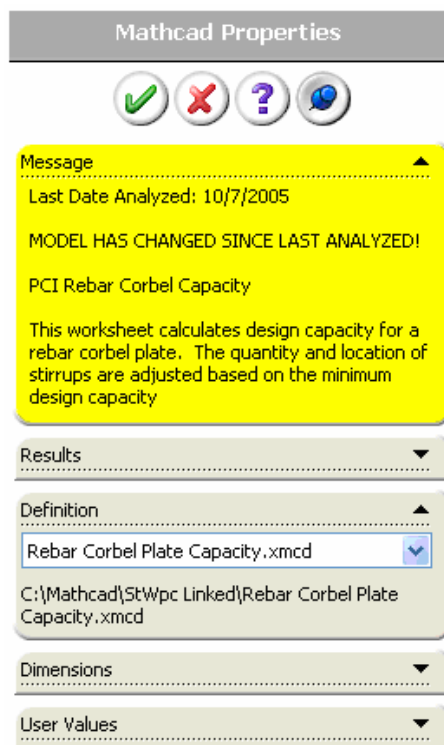


Figure 4 - StructureWorks Precast Mathcad Property Page

The Property Page resides on the left side of the screen allowing as much model interaction as possible. This provides ease of variable checking and adding dimensional names.

From Figure 4 the Mathcad Properties page contains the following:

- Message:** Indicating the last date of the analysis. A message is shown if the model has been changed since the last time it was analyzed. Below the warning message is a title and description of the worksheet that is in the que to be run. The title and description are 100% user defined utilizing the native Windows file description object.
- Results:** The results section is a multi line return from Mathcad. Any concatenated string can be returned and listed here.
- Definition:** This section provides a drop down list of all the available Mathcad worksheets in the design binder. The path of the current selection is shown.
- Dimension:** This section contains all the dimensional values that are linked between StructureWorks Precast the current Mathcad 13 worksheet. In addition to the dimension names, the section also contains the current dimension values.
- User Values:** This section contains the additional values required for the design or analysis. This allows other inputs to be set without the need to tab between the applications.

Integration Samples

To demonstrate the process we will use a Rebar Corbel. In the first pass we investigate the concept of Capacity Checking and the second pass we investigate Automated Design. Capacity checking entails creating a model and determining the maximum design load, ϕV_n , based on the current model and some engineering parameters. Automated Design will automatically change the model, I.E. bar size, spacing, hook requirements, and weld based on a given set of parameters and the applied load, V_u .

Capacity Check

The following Rebar Corbel is modeled with the following geometry data listed in the property page on the left of the model in the Dimension Section. The User values are listed below in the User Value section. Even though any variable can be transferred between the applications, the engineering parameters that are required, f'_c , E_{st} , etc., are stored in the Mathcad Worksheet.

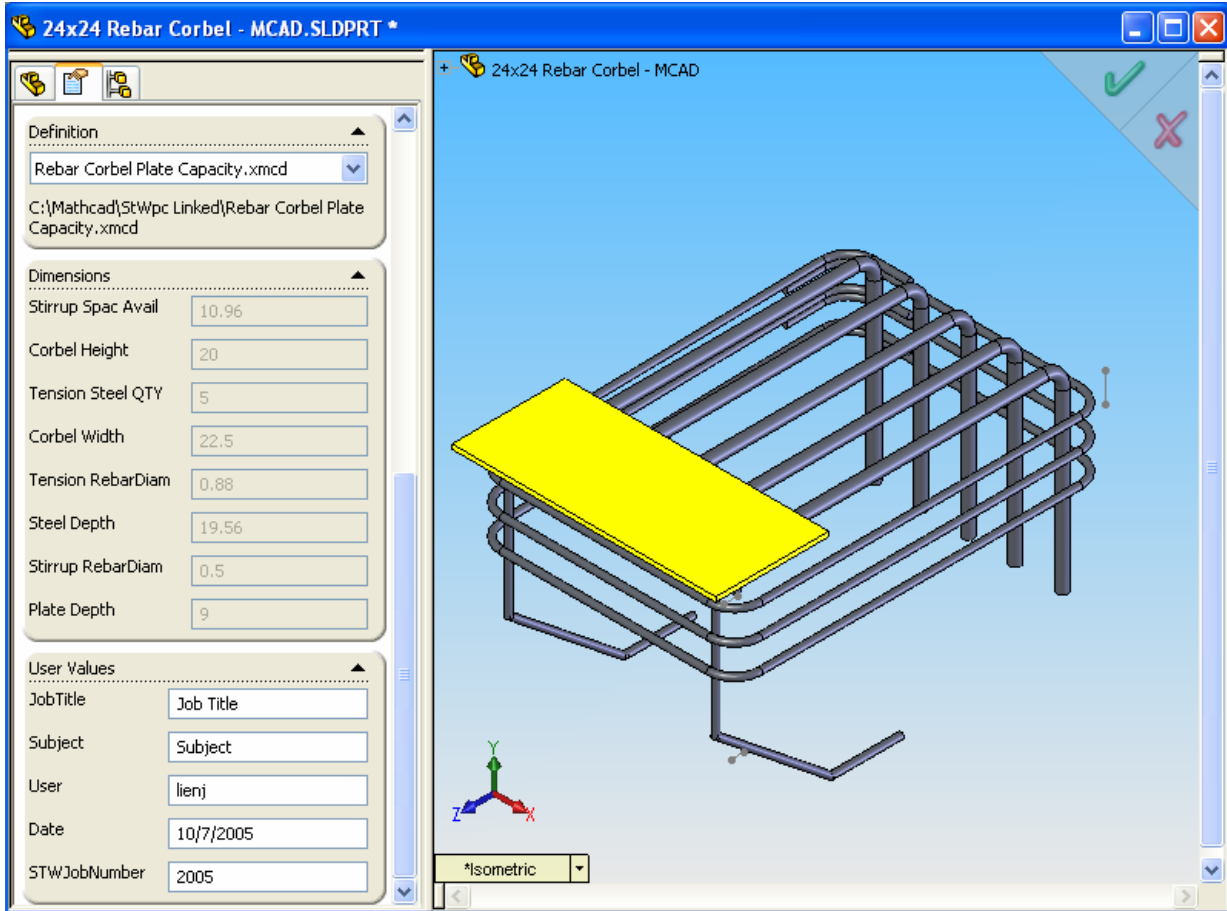


Figure 5 – Capacity Check Rebar Corbel Example

The Mathcad worksheet selected for the Analysis is *Rebar Corbel Plate Capacity.XMCD*.

On the Mathcad Properties page the dimensions and user values are listed. When the worksheet is calculated (the Green Check) the results section is populated with a user defined array in Mathcad.

Automated Rebar Corbel Design

Even though the model is the exact same, the Property Page shows more dimensional control and user values. New dimensions are sent to Mathcad for the automated layout routine. They include the Plate Width, Edge Clearance, and available Hook length. New User variables are introduced for Preferred Rebar Size, Minimum and Maximum Spacing, in addition to an applied Ultimate Load.

Mathcad Properties



Message

Last Date Analyzed: 10/7/2005

MODEL HAS CHANGED SINCE LAST ANALYZED!

PCI Rebar Corbel Design

This Mathcad sheet calculates the tension steel, A_s , tension spacing, stirrup steel, A_h , and stirrup spacing required for a rebar corbel given a Shear force and corbel dimensions.

Results

Corbel Tension Steel = (5) #6

Tension Steel Spacing = 2.25 in on center

Min Hook Length = 12.73 in

Corbel Stirrup Steel = (2) #4

Shear Steel Spacing - (1) at 2 in and remaining (1) at 7.024 in on center

$V_{max} = 440.156$ kips

Definition

Rebar Corbel Plate Design.xmcd

C:\Mathcad\StWpc Linked\Rebar Corbel Plate Design.xmcd

Dimensions

Hook

Stirrup Spac Avail

Corbel Height

Corbel Width

Steel Depth

Plate Width

Plate CLR

Plate Depth

User Values

JobTitle

Subject

User

Date

STWJobNumber

Applied Ultimate Load (kip)

Tension RebarDiam Pref

Stirrup RebarDiam Pref

SpaceMin (in)

SpaceMax (in)

Figure 6 – Complete Automated Design Property Page

Automated Stair Design

Automated design has had limited success. Much of this is due to the complexity of what should be included in the design and how detailed and company specific can the output be. When one uses an application where the user has full control of all parameters, the user can create the custom design applications based on as few or as many design parameters. Within the Mathcad Worksheet the user can save standard values and only pass from StructureWorks Precast the required values for additional design data. The Worksheet in Appendix D is a program to design Precast Stair Units for Flexure.

Standard Variables

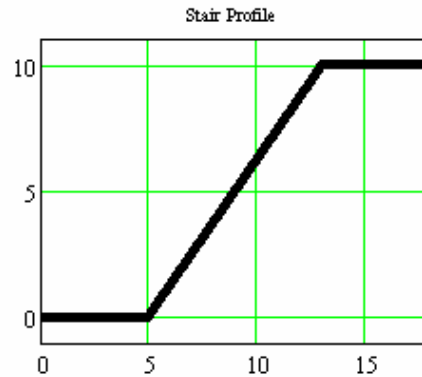
The standard options for Mesh in Tension and Compression in addition to the percentage of Tension Rebar used in Compression for Displacement control are all save in the worksheet. The yellow regions are standard options and for convenience the light blue regions are user inputs from StructureWorks Precast. See Figure 7.

Automated Design Information

Tension Steel

Minimum Bar Spacing $Spac_{min} := 2\text{-in}$
 Maximum Bar Spacing $Spac_{max} := 8\text{-in}$
 Edge Clearance $CLR_{edge} := 2.5\text{-in}$

Preferred Rebar Size



Tension Mesh Size
 Number of Layers

Longitudinal Size and Space

Transverse Size and Space

Deflection Control Steel

Include Compression Mesh
 Yes
 No

Yes = 1 Layer of the Tension Mesh

Percent of Tension Steel as Compression Steel $\%CompressionSteel :=$

$\%CompressionSteel = 0\%$

A minimum of 2 trim bars will be provided as compression steel

If Upper Landing Steel requirements govern over supplied $\%CompressionSteel$ then Compression Steel in the model is based on the landing tension requirement layout.

Figure 7 - Standard Automated Design Parameters

Top-Down Precast Dimensions

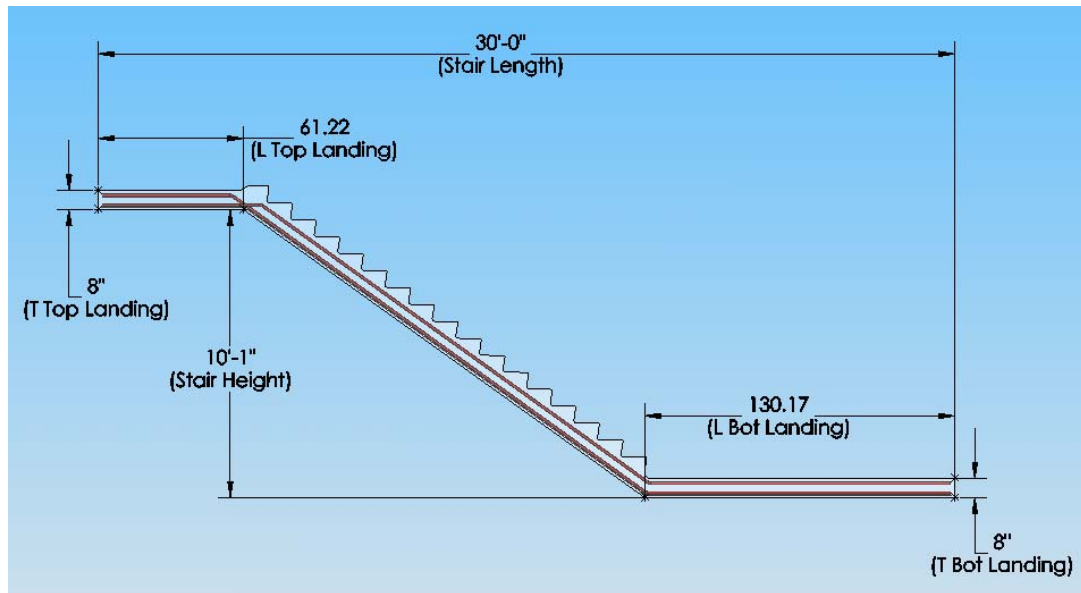


Figure 8 – Top-Down Reference Design Parameters

Conclusion

Buildings are complex. Engineering and designing them inherently contains risk. The integration presented in this paper allows the design engineer to develop and use current methods that many are comfortable with. Black box solutions have always been slow to be adopted while this method although the first of its kind, allows the user to expose as much or as little design validation as they choose. Checking the data transfer and adjusting the dimensional values required for the integration, requires limited effort on the part of the designer.

StructureWorks Precast has been shown to reduce risk across the entire enterprise; this is one additional area for greater returns and increased productivity. The transition from 2-D to 3-D is a difficult one, but by selecting the proper tool the transition can be simple, fast, and beneficial.

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Appendix A

Rebar Corbel Plate Capacity Mathcad Worksheet



Rebar Corbel Capacity

Corbel Height	CorbelHeight := 20·in	Quantity of Tension Bars	SteelQTY := 5
Corbel Width	CorbelWidth := 22.5·in	Diameter of Tension Bars	SteelRebarDiam := 0.875·in
Effective Depth	SteelDepth := 19.562·in	Diameter of Shear Stirrups	StirrupRebarDiam := 0.500·in
Plate Depth	PlateDepth := 9·in		

Input

Load Ratio	Ratio := 20%	Material Data	$\phi := .85$	$\lambda := 1.0$	$f_y := 60\text{·ksi}$	$f'_c := 5000\text{psi}$
Load Eccentricity	$e := \text{PlateDepth} \cdot 75\%$		$\mu := 1.4$	(Monolithic corbels)		

Calculations

Area of Crack Interface $A_{cr} = 440.145 \text{ in}^2$

Total Tension Steel $A_s = 3 \text{ in}^2$

Maximum Shear

$$\mu \lambda f'_c = 1000 \text{ psi}$$

$$\phi V_{n1} := \mu \lambda f'_c (\lambda^2 \cdot A_{cr}) \quad \phi V_{n1} = 440.145 \text{ kip}$$

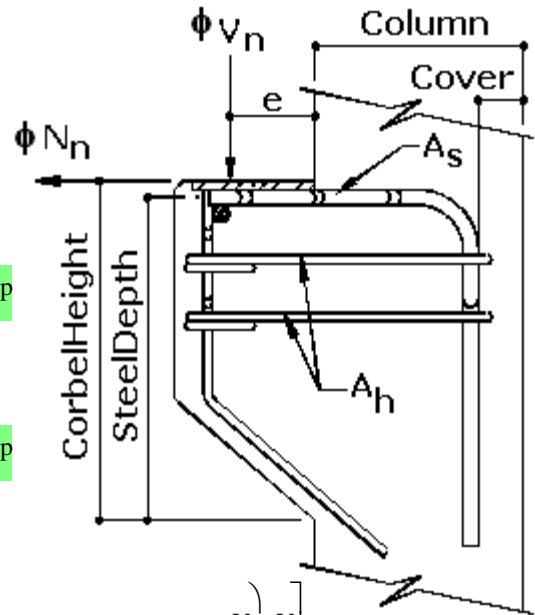
Capacity Based on Cantilever Bending

$$\phi V_{n2} := \frac{\phi \cdot A_s \cdot f_y \cdot d}{e + \text{Ratio} \cdot (H - d) + \text{Ratio} \cdot d} \quad \phi V_{n2} = 278.417 \text{ kip}$$

Capacity Based on Shear Friction

$$\phi V_{n3} := \text{root} \left[\left(\frac{\phi \cdot A_s \cdot f_y}{2} - V \right), V \right] \quad \phi V_{n3} = 294.819 \text{ kip}$$

$$3 \cdot \text{if} \left(\frac{1000 \cdot \text{psi} \cdot \lambda \cdot A_{cr} \cdot \mu}{V} > 3.4, 3.4, \frac{1000 \cdot \text{psi} \cdot \lambda \cdot A_{cr} \cdot \mu}{V} \right) + \text{Ratio}$$



PCI Equation 6.8.6

$$A_h := .5 \cdot \left(A_s - \frac{\text{Ratio} \cdot \min(\phi V_n)}{\phi \cdot f_y} \right)$$

PCI Equation 6.8.5 - Minimum Area of Steel

$$A_{s\min} = 1.467 \text{ in}^2$$



Output

Design Capacity

$$\phi V_n = 278.42 \text{ kip}$$

Quantity of Stirrups required for Design Capacity

$$\text{QTYStirrup} = 3$$

Amount of Horizontal Tie steel required for design capacity

$$A_h = 0.954 \text{ in}^2$$

Horizontal Ties Should be distributed within the top $\frac{2}{3} \cdot d = 13.041 \text{ in}$ of the main tension steel





Appendix B

Automated Stair Unit Design Mathcad Worksheet



Job Title Job Title By Eng Job Number Job#
 Subject Subject Date 10/10/2005 Sheet of

▢ **Stair Dimensions**

Landing Lengths

$L_{\text{landing_top}} := 60 \cdot \text{in}$

$L_{\text{landing_bottom}} := 60 \cdot \text{in}$

Landing Thickness

$t_{\text{landing_top}} := 8 \cdot \text{in}$

$t_{\text{landing_bottom}} := 8 \cdot \text{in}$

Stair Width

Width := 60·in

Total Elevation Change

Height := 121·in

Total Horizontal Length

Length := 18·ft

Load and Steel Design Parameters

ACI Design Code

DC := "318-99"

Compression Block Width

b := Width

Landing Topping Thickness

Land_{Top} := 2·in

Effective Steel Depth

d := 6.5in

Live Load

P_{LL} := 100·psf

Steel Yield Strength

f_y := 60·ksi

% Live Load Sustained

%_{LL} := 10%

Concrete Strength

f'_c := 5000·psi

Concrete Unit Weight

γ_c := 150·pcf

Section Height

h := 8.5·in

▢ Steel Modulus

E_{st} := 29000·ksi

Concrete Modulus

E_c = 4286.826 ksi

Step Section Area (7x11)

A_{step} := 141.35·in²

▢ **Automated Design Information**

Tension Steel

Minimum Bar Spacing

Spac_{min} := 2·in

Maximum Bar Spacing

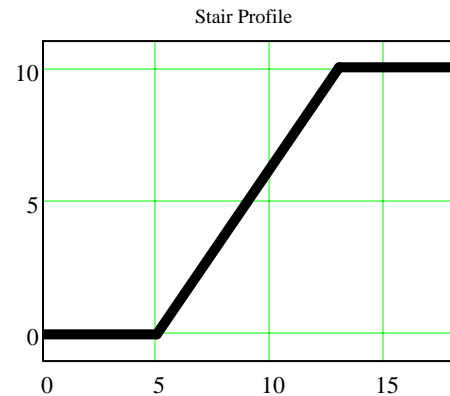
Spac_{max} := 8·in

Edge Clearance

CLR_{edge} := 2.5·in

Preferred Rebar Size

#7
#8
#9
#10



Tension Mesh Size

Number of Layers

2
3
4

Longitudinal Size and Space

W3
W2.9
W2.5

8"
10"
12"

Transverse Size and Space

W3.5
W3
W2.9

8"
10"
12"

Deflection Control Steel

Include Compression Mesh

Yes
 No

Yes = 1 Layer of the Tension Mesh

Percent of Tension Steel as Compression Steel

%CompressionSteel :=

%CompressionSteel = 0 %

0

A minimum of 2 trim bars will be provided as compression steel

If Upper Landing Steel requirements govern over supplied %CompressionSteel then Compression Steel in the model is based on the landing tension requirement layout.



Job Title	Job Title	By	Eng	Job Number	Job#
Subject	Subject	Date	10/10/2005	Sheet	of

End Reactions

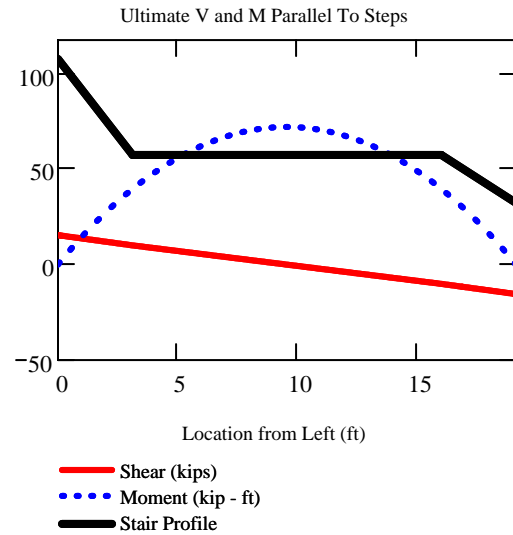
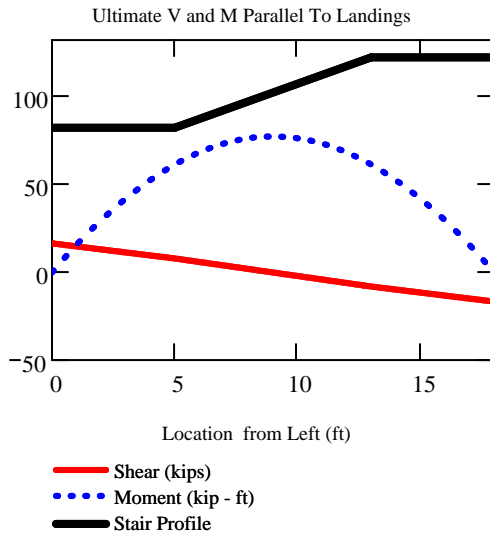
Left Working $\max(R_L^{(1)}) = 10.938 \text{ kip}$

Right Working $\max(R_R^{(1)}) = 10.938 \text{ kip}$

Left Ultimate $\max(R_L^{(2)}) = 16.663 \text{ kip}$

Right Ultimate $\max(R_R^{(2)}) = 16.663 \text{ kip}$

Shear and Ultimate Moment Diagrams



Maximum Moments and Steel Requirements

Ultimate

Parallel To Landing $M_{\max_{1,2}} = 933.95 \text{ kip}\cdot\text{in}$

Location $X_{\max_{1,2}} = 9 \text{ ft}$

Parallel To Stairs $M_{\max_{2,2}} = 867.249 \text{ kip}\cdot\text{in}$

Location $X_{\max_{2,2}} = 9.543 \text{ ft}$

Steel Requirements

$$\text{MainRequirements} = \left(\begin{array}{l} \text{"2.803 sq. in Flexural Steel Required"} \\ \text{"0.9 Strength Reduction Factor"} \\ \text{"13.08 sq. in Balanced Steel"} \\ \text{"9.81 sq. in Maximum Steel based on 75\% of Balanced Steel"} \\ \text{"1.379 sq. in Minimum Steel Required"} \\ \text{"0.918 sq. in Longitudinal Shrinkage and Temperature Steel Required"} \end{array} \right)$$

Top Landing Moments at Landing Stair Interface

Parallel To Landing $M_{\text{Landing_max}_1} = 741.039 \text{ kip}\cdot\text{in}$

Parallel To Stairs $M_{\text{Landing_max}_2} = 476.923 \text{ kip}\cdot\text{in}$

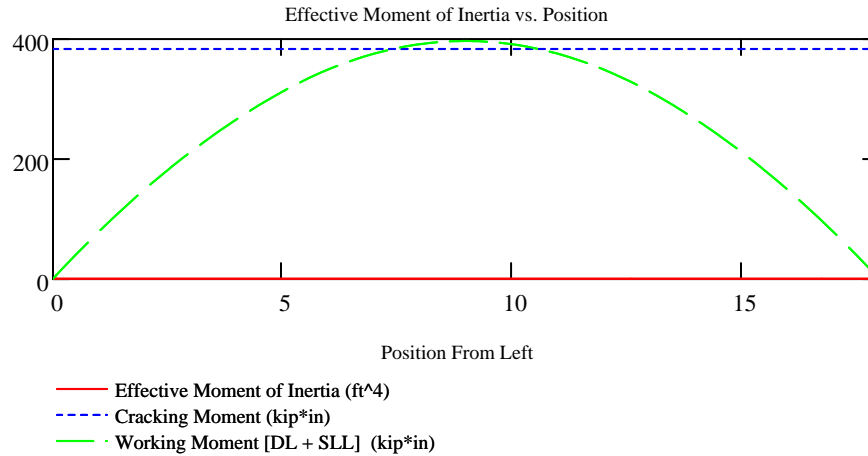
Steel Requirements

$$\text{LandingRequirements} = \left(\begin{array}{l} \text{"2.199 sq. in Flexural Steel Required"} \\ \text{"0.9 Strength Reduction Factor"} \\ \text{"13.08 sq. in Balanced Steel"} \\ \text{"9.81 sq. in Maximum Steel based on 75\% of Balanced Steel"} \\ \text{"1.379 sq. in Minimum Steel Required"} \\ \text{"0.918 sq. in Longitudinal Shrinkage and Temperature Steel Required"} \end{array} \right)$$

Job Title	Job Title	By	Eng	Job Number	Job#
Subject	Subject	Date	10/10/2005	Sheet	of

▢ **Deflections Based on Profile Parallel to Landing**

- Gross Moment of Inertia $I_o(b, h) = 3070.625 \text{ in}^4$
- Cracked Transformed Moment of Inertia $I_{cr}(b, d, E_{st}, f'_c, \gamma_c, A_{Total_Supplied}) = 1235.548 \text{ in}^4$
- Cracking Moment $M_{cr}(f'_c, b, h) = 383.163 \text{ kip}\cdot\text{in}$
- Effective Moment of Inertia as a function of Position (through applied Moment)



- Location of Maximum Deflection $\xi = 9 \text{ ft}$ **Long Term Modifier** $\lambda = 1.592$
- Initial Deflection Due to DL and Sustained LL $\delta(\xi) = 0.147 \text{ in}$
- Final Deflection @ Due to DL and Sustained LL $\delta(\xi) + \lambda \cdot \delta(\xi) = 0.381 \text{ in}$
- Deflection Limits $\frac{L_1}{360} = 0.6 \text{ in}$

Final Steel Design

Required Tension Steel $A_{sdesign} = 2.803 \text{ in}^2$

Tension Steel Design

$A_{Total_Supplied} = 8.619 \text{ in}^2$ TensionSteel = "(8) #9 @ 7.857" on center" $A_{rb_Supplied} = 8 \text{ in}^2$

TensionMesh = "(3) Layer of 8 x 8 - W3 x W3.5" $A_{mesh_Supplied} = 0.619 \text{ in}^2$

Compression Steel Design

$A_{cTotal_Supplied} = 2 \text{ in}^2$ CompressionMesh = "No Compression Mesh in Design" $A_{c_rb_Supplied} = 2 \text{ in}^2$

CompressionSteel = "(2) #9 @ 55" on center" $A_{c_mesh_Supplied} = 0 \text{ in}^2$

Required Landing Tension Steel $A_{sLandingdesign} = 2.199 \text{ in}^2$

Landing Steel Design

LandingSteel = "(8) #9 @ 7.857" on center" $A_{landing_Supplied} = 8 \text{ in}^2$

Stair Section Design Summary

Tension Steel (GREEN)

(8) #9 @ 7.857" on center

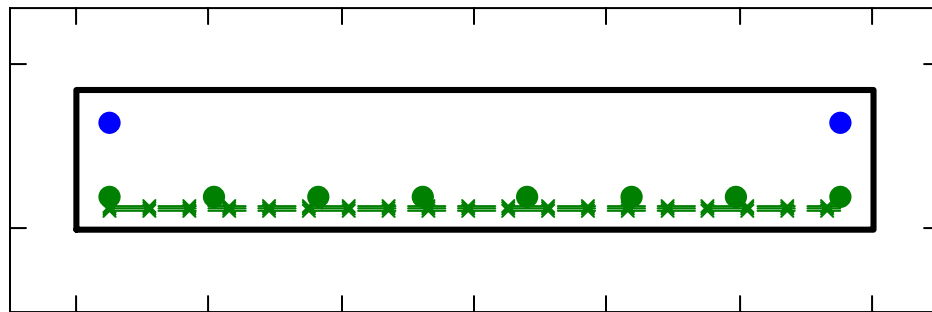
(3) Layer of 8 x 8 - W3 x W3.5

Compression Steel (BLUE)

(2) #9 @ 55" on center

No Compression Mesh in Design

Cross Section / Rebar Layout

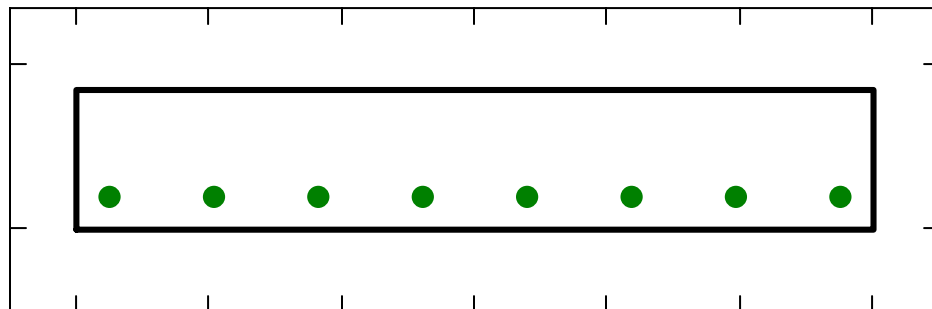


Landing Section Design Summary

Landing Steel (if Required)

(8) #9 @ 7.857" on center

Cross Section / Rebar Layout



Concrete Properties

28 Day Strength $f'_c = 5000 \text{ psi}$

Release Strength $f_{ci} := 2500 \text{ psi}$

